July 20, 2023

Bonnie Schwartz
Pound Ridge Library
271 Westchester Avenue
Pound Ridge, NY 10576

RE: Traffic Study<br>Pound Ridge Library<br>271 Westchester Avenue (NY Route 137)<br>Town of Pound Ridge, Westchester County, NY

Dear Bonnie:

## INTRODUCTION

DTS Provident Design Engineering, LLP (DTS Provident), a licensed Professional Engineering firm in the State of New York, has prepared this Traffic Study to evaluate the traffic and safety as well as potential proposed additional parking at the existing Pond Ridge Library located at 271 Westchester Avenue (NY Route 137) in the Town of Pound Ridge, Westchester County, New York. A map of the Site Location is illustrated on Figure No. 1 in Appendix B.

DTS Provident has been retained to analyze the traffic associated with the library including a potential increase in the number of parking spaces and to identify improvements, if any, to mitigate any adverse impact. This Traffic Study has been prepared using industry-standard traffic engineering procedures to document the findings and conclusions of the analysis undertaken to measure the traffic impacts associated with the library and potential modifications.

To perform this Study, traffic counts were performed, speed measurements were conducted, sight distance was measured, capacity analyses were conducted, and crash history was reviewed. The following is a summary of our findings and conclusions.

## EXISTING CONDITIONS

Pound Ridge Library is located along Westchester Avenue (NY Route 137) at its intersection with Salem Road (NY Route 124) and Stone Hill Road (NY Route 137) in the Town of Pound Ridge. The intersection is a triangular intersection with a landscaped island in the middle. Both NY Route 137 and NY Route 124 consist of one lane per direction with minimal shoulders. There are no sidewalks in the area. There is a Stop sign facing the westbound approach of NY Route 124 as well as the southwest bound approach of NY Route 137 (Stone Hill Road).

The library is accessed by a paved driveway that wraps around to the back of the building. Various parking spaces are laid out surrounding the site. A gravel path is located at the back of the site that goes through a wooded area and eventually leads to a few residential houses.

## EXISTING TRAFFIC COUNTS

In order to analyze the traffic impacts associated with the library, DTS Provident first determined the existing traffic volume conditions. As the library is not open during the Peak AM Roadway Hour (8:00 AM to 9:00 AM), the traffic counts focused on the Peak PM Roadway Period, the busiest period on the roadway. Thus, representatives of DTS Provident conducted turning movement traffic counts on Tuesday June 20, 2023, from 4:00 PM to 6:00 PM. Based upon the traffic counts conducted, the Peak Roadway Hour was determined to be 5:00 PM to 6:00 PM.

In addition to the turning movement counts at the intersection, automatic traffic recorder (ATR) counts were performed along Westchester Avenue for a one-week period, collecting traffic volumes and speeds along the roadway in both directions.

The 2023 Existing Traffic Volumes are illustrated on Figure No. 2 in Appendix B. The ATR counts and data are contained in Appendix E.

## TRIP GENERATION AND FUTURE TRAFFIC VOLUME CONDITIONS

Upon establishing the existing traffic volume conditions, DTS Provident grew the existing volumes to the future design year of 2025 using a growth rate of $1.5 \%$ per year compounded annually. This growth rate was determined by analyzing NYSDOT traffic growth on Westchester Avenue (NY Route 137) and Stone Hill Road (NY Route 137) over the past several years. These grown volumes form the 2025 NoBuild Traffic Volumes which are illustrated on Figure No. 3 in Appendix B.

At this time the library is considering adding additional parking. Although the library does not generate significant traffic, DTS Provident added an additional 10 entering trips and 10 exiting trips to account for the possibility of increased traffic. The vehicular trips were applied to the site driveway and adjacent roads to form the 2025 Build Traffic Volumes which are illustrated on Figure No. 4 in Appendix B.

## TRAFFIC CAPACITY ANALYSIS

DTS Provident utilized Synchro software to calculate the capacity analysis of the intersection of the Site Driveway and Westchester Avenue/Stone Hill Road (NY Route 137) \& Salem Road (NY Route 124) for the Existing, No-Build, and Build traffic volume conditions. It should be noted that the intersection currently operates as a 3-way stop which is an atypical intersection control for a four-legged intersection and thus is not supported by Highway Capacity Manual (HCM) methodology. In order to evaluate the intersection, DTS Provident conducted traffic simulations within Synchro and used the simulation traffic
report feature to obtain levels of service (LOS) and delays (seconds per vehicle). A Level of Service 'A' represents the best roadway operating conditions while a Level of Service ' $F$ ' represents the worst roadway operating conditions. A description of Level of Service Standards is contained in Appendix A. Table No. 1 below summarizes the capacity analysis results for the Site Driveway.

| TABLE NO. 1 |  |  |
| :---: | :---: | :---: |
| OVERALL LEVEL OF SERVICE SUMMARY |  |  |
| Intersection | PEAK WEEKDAY PM HOUR |  |
|  | No-Build | Build |
|  | LOS | LOS |
| Site Driveway/Stone Hill Road (Route 137) \& | Delay (sec/veh) | Delay (sec/veh) |
| Westchester Avenue/Salem Drive (Route 124) | a | b |

Notes:
Levels of Service for unsignalized intersections are denoted by lowercase letters.
Average delay is represented in seconds per vehicle.
Delay is shown for the worst side-street approach
As shown in Table No. 2 above, Levels of Service and delays for the Site Driveway are expected to operate appropriately with minimal delays. A more detailed Level of Service Table is contained in Appendix C and copies of the capacity analysis summary sheets are contained in Appendix D.

## SIGHT DISTANCE

DTS Provident conducted a sight distance analysis for the Site Driveway and the intersection. Below is a summary table of the analysis.

| TABLE NO. 2 <br> SIGHT DISTANCE SUMMARY <br> LIBRARY DRIVEWAY AND NY ROUTE 137 |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| DESCRIPTION | FIELD <br> MEASUREMENT | AASHTO <br> RECOMMENDED/ REQUIRED SIGHT DISTANCE FOR 30 MPH (Adjusted for Grade where stated | AASHTO <br> RECOMMENDED/ REQUIRED SIGHT DISTANCE FOR 35 MPH <br> (Adjusted for Grade where stated) | AASHTO RECOMMENDED/ REQUIRED SIGHT DISTANCE FOR 40 MPH (Adjusted for Grade where stated) |
| Driveway Exiting sight line looking right (onto Salem Road) to the approaching vehicle | 300 | $\begin{gathered} 335 \\ \text { (if no Stop sign) } \end{gathered}$ | 390 <br> (if no Stop sign) | 445 <br> (if no Stop sign) |
| Driveway Exiting sight line looking straight (onto Stone Hill Road) to the approaching vehicle | 250 | 335 <br> (if no Stop sign) | $\begin{gathered} 390 \\ \text { (if no Stop sign) } \end{gathered}$ | 445 <br> (if no Stop sign) |
| Driveway Exiting sight line looking left (onto Westchester Avenue) to the approaching vehicle | 200 | $335$ | $390$ | 445 |
| Rear end sight line (along Salem Road) for the left turn entering vehicle from a vehicle approaching from the same direction along Salem Road | 570 | $\begin{gathered} 200 \\ \text { (Stopping Sight } \\ \text { Distance) } \end{gathered}$ | $\begin{gathered} 250 \\ \begin{array}{c} \text { (Stopping Sight } \\ \text { Distance) } \end{array} \end{gathered}$ | $\begin{gathered} 305 \\ \begin{array}{c} \text { (Stopping Sight } \\ \text { Distance) } \end{array} \end{gathered}$ |
| Rear end sight line (along Stone Hill Road for the left turn entering vehicle from a vehicle approaching from the same direction on Stone Hill Road | 200 | 200 <br> (Stopping Sight Distance) | $\begin{gathered} 250 \\ \text { (Stopping Sight } \\ \text { Distance) } \end{gathered}$ | $\begin{gathered} 305 \\ \text { (Stopping Sight } \\ \text { Distance) } \end{gathered}$ |
| Sight line from the left turn vehicle on Salem Road entering driveway to a vehicle approaching from the opposite direction on Westchester Avenue | 370 | 245 | 285 | 325 |
| Rear end sight line for the right turn vehicle entering driveway or vehicle exiting driveway from a vehicle approaching from the same direction along Westchester Avenue | $\begin{gathered} 335 \\ \text { (requires } \\ \text { vegetation } \\ \text { clearance) } \end{gathered}$ | $\begin{gathered} 215 \\ \text { (Stopping Sight } \\ \text { Distance -adjusted } \\ \text { for grade) } \end{gathered}$ | 271 <br> (Stopping Sight Distance <br> - adjusted for grade) | 333 <br> (Stopping Sight Distance - adjusted for grade) |

* Sight Distance Data obtained from field measurements by representatives of DTS Provident on 07/05/2023
*AASHTO requirements based upon "A Policy on Geometric Design of Highways and Streets" 2018, $7^{\text {th }}$ Edition

The posted speed limit for the northbound direction of Westchester Avenue (NY Route 137) in the vicinity of the library driveway is 30 mph while the southbound direction speed limit is also 30 mph . North and east of the driveway, there are higher speed limits.

In the northbound direction of Westchester Avenue approaching the library driveway, there is a yellow warning sign indicating the "Library Driveway Ahead" as well as a 25 mph Curve Warning sign. Speed measurements conducted for the northbound direction determined an Average Speed of 34 mph and an $85^{\text {th }}$ Percentile Speed of 39 mph , thus most vehicles are traveling greater than the speed limit. For the southbound direction, speed measurements indicated an Average Speed of 30 mph and an $85^{\text {th }}$ Percentile Speed of 34 mph . Thus, for analysis purposes, $30 \mathrm{mph}, 35 \mathrm{mph}$ and 40 mph were reviewed.

The key sight distance is between the combination of vehicles traveling northbound on Westchester Avenue and vehicles exiting the driveway looking left. The sight distance recommendations and minimum sight distance requirements based on these speed limits were obtained from American Association of State Highway and Transportation Officials (AASHTO) "A Policy on Geometric Design of Highways and Streets" 2018, $7^{\text {th }}$ Edition. Based on these recommendations and requirements, as well as accounting for the $-6 \%$ grade on a portion of Westchester Avenue, although the "recommended" intersection sight distance is not met for the driveway, particularly at 40 mph , the "required" sight distance, the Stopping Sight Distance, is met. Thus, the minimum required sight distances are met. However, it is strongly recommended that vegetation be continuously cleared within the right-of-way in the area west of the driveway to provide this sight distance for vehicles exiting the driveway looking to the left. Drivers exiting the library have to pull up closer to the intersection to see to the left. In addition to the vegetation, there is a wall, trees, utilities poles and other items limiting the sight distance along with a curve in the roadway.

It is noted that vehicles approaching in the southbound direction on both Westchester Avenue (NY Route 124) and Stone Hill Road (NY Route 137) have stop signs and thus these vehicles stop before travelling through the intersection. If the stop signs did not exist, then these movements would have sight distance limitations due to the roadway curvature and vegetation including the large tree and fence to the east of the driveway.

## CRASH DATA

DTS Provident obtained crash data in the vicinity of the library from the New York State Department of Transportation for the last three available years. There was only one recorded crash at the intersection during this time period. This crash was a head-on collision amongst two motor vehicles on September 5,2020 at $4: 17$ PM under dry conditions and resulted in two injuries. The crash occurred on the Stone Hill Road portion of the intersection near Salem Road and thus did not involve the library driveway. The apparent contributing factor for the crash was that one of the vehicles was passing or "lane usage improperly".

Based on only one crash occurring at the intersection over a three-year period, there is not a high crash rate at this intersection. Some other crashes occurred near the intersection but were also not related to the driveway.

## CONCLUSION

Based upon the field observations and information contained herein, it is the considered professional opinion of DTS Provident that the traffic associated with the library and a possible increase in the number of parking spaces will not have an adverse impact upon the adjacent roadway network. Although the minimum sight distance is met, it is strongly recommended that vegetation be continuously cleared within the right-of-way in the area to the west of the driveway to provide this sight distance for vehicles exiting the driveway looking to the left.

Very truly yours,

## DTS Provident Design Engineering, LLP

Brian Dempsey P.E., PTOE, RSP1
Partner

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APPENDIX A
LEVEL OF SERVICE STANDARDS

## 1. LEVEL OF SERVICE

## CONCEPT

The Highway Capacity Manual, published by the Transportation Research Board of the U.S. Government, established a system by which highway facilities are examined for their adequacy to handle traffic volumes. The terminology "Level of Service" is used to provide a "qualitative" evaluation based on certain "quantitative" calculations which are related to empirical values.

Intersection Capacity, Delay and resultant Levels of Service are dependent upon a number of factors, including the following:

- Area Type
- Intersection geometrics
- Traffic volumes
- Parking conditions
- Pedestrian activity
- Vehicle Mix
- Bus Stop location and activity
- Peak Hour Factor
- Traffic Signal operation, if applicable

Ramp and weaving area Densities and resultant Levels of Service are dependent upon a number of factors, including the following:

- Number of lanes
- Configuration of weaving area
- Length of acceleration/deceleration lanes
- Vehicle speeds
- Traffic volumes
- Vehicle Mix
- Peak Hour Factor


## FACTORS

## SIGNALIZED INTERSECTIONS

Level of Service for Signalized Intersections is defined in terms of Delay, which is a measure of driver discomfort, frustration, fuel consumption, and loss of travel time. Specifically, Level of Service criteria are stated in terms of the Average Control Delay per vehicle for the peak 15-minute period within the hour analyzed.

Delay is a complex measure and is dependent upon a number of variables, including:

- Cycle length
- $\quad$ Ratio of Green time to Cycle length (G/C)
- $\quad$ Ratio of Volume to Capacity (V/C) for lane group or approach
- Traffic signal progression


## UNSIGNALIZED INTERSECTIONS

Level of Service for Unsignalized Intersections is also defined in terms of Delay. The amount of Delay is based upon the availability of "gaps" in the mainline traffic stream and the acceptance of these gaps by motorists waiting on the side street to enter the main street traffic flow.

## RAMP AND RAMP JUNCTIONS

Level of Service for ramp freeway junctions and the ramp proper are defined in terms of Density (passenger cars per mile per lane). Density is related to the traffic flow in the area of influence.

## WEAVING AREAS

Level of Service for weaving areas is defined in terms of Density (passenger cars per mile per lane). Density is based on the ratio of weaving vehicles to non-weaving vehicles and on vehicle speeds in the weaving area of influence

## CRITERIA

The criteria for the various Level of Service designations are as follows:

|  | SIGNALIZED | UNSIGNALIZED |
| :---: | :---: | :---: |
| LEVEL OF <br> SERVICE | Average Control Delay <br> per Vehicle (Seconds) | Average Control Delay <br> per Vehicle (Seconds) |
| A | 10.0 or less | 10.0 or less |
| B | 10.1 to 20.0 | 10.1 to 15.0 |
| C | 20.1 to 35.0 | 15.1 to 25.0 |
| D | 35.1 to 55.0 | 25.1 to 35.0 |
| E | 55.1 to 80.0 | 35.1 to 50.0 |
| F | 80.1 or greater | 50.1 or greater |


| Level of Service | Ramp-Freeway Junction | Ramp Proper | Weaving Areas |  |
| :---: | :---: | :---: | :---: | :---: |
|  | Maximum Density $\mathbf{p c} / \mathbf{m i} / \mathbf{l n}$ | Density Range pc/mi/ln | Maximum Density pc/mi/ln |  |
|  |  |  | Freeway Weaving Area | Multi-lane + C-D Weaving Area |
| A | $\leq 10$ | $\leq 11$ | $\leq 10$ | $\leq 12$ |
| B | $>10-20$ | $>11-18$ | $>10-20$ | $>12-24$ |
| C | >20-28 | $>18-26$ | > 20-28 | >24-32 |
| D | $>28-35$ | $>26-35$ | $>28-35$ | >32-36 |
| E | >35 | >35-45 | >35-43 | >36-40 |
| F | Demand exceeds capacity | >45 | $>43$ | $>40$ |

## DESCRIPTION

The following is a brief description of each of the six Level of Service designations as defined by the Highway Capacity Manual:

## SIGNALIZED INTERSECTIONS

## LEVEL OF SERVICE A

Average Control Delay - 10.0 secs. or less
Describes operations with very low delay. Occurs when progression is extremely favorable and most vehicles arrive during the Green Phase and do not stop at all. Short cycle lengths may also contribute to low delay.

## LEVEL OF SERVICE B

Average Control Delay - 10.1 to 20.0 secs.
Generally occurs with good progression and/or short cycle lengths. More vehicles stop than for Level of Service A, causing higher levels of average delay.

## LEVEL OF SERVICE C

Average Control Delay - 20.1 to 35.0 secs.
Higher delays may result from fair progression and/or longer cycle lengths. Individual cycle failures may begin to appear at this Level of Service. The number of vehicles stopping is significant, although many still pass through the intersection without stopping.

## LEVEL OF SERVICE D

Average Control Delay - 35.1 to 55.0 secs.
The influence of congestion becomes more noticeable. Longer delays may result from some combination of unfavorable progression, long cycle lengths, or high Volume/Capacity (V/C) Ratios. Many vehicles stop, and the proportion of vehicles not stopping declines. Individual cycle failures are noticeable.

## LEVEL OF SERVICE E

Average Control Delay - 55.1 to 80.0 secs.
The limit of acceptable delay.
Higher delay values generally indicate poor progression, long cycle lengths, and high V/C Ratios. Individual cycle failures are frequent occurrences.

## LEVEL OF SERVICE F

Average Control Delay - in excess of 80.0 secs.
Unacceptable to most drivers.
Occurs with oversaturation, i.e., arrival flow rates exceed the capacity of the intersection. May also occur at high V/C Ratios below 1.0 with many individual cycle failures. Poor progression and long cycle lengths may also be major contributing factors.

## UNSIGNALIZED INTERSECTIONS

## LEVEL OF SERVICE A

Average Control Delay - 10.0 secs. or less
Operations with little or no delay to minor turning movements.

## LEVEL OF SERVICE B

Average Control Delay - 10.1 to 15.0 secs.
Operations with short delays on minor turning movements.

## LEVEL OF SERVICE C

Average Control Delay - 15.1 to 25.0 secs.
Operations with average delays on minor turning movements.

## LEVEL OF SERVICE D

Average Control Delay - 25.1 to 35.0 secs.
Operations with some delays on minor turning movements.

## LEVEL OF SERVICE E

Average Control Delay - 35.1 to 50.0 secs.
Operations with long delays on minor turning movements.

## LEVEL OF SERVICE F

Average Control Delay - In excess of 50.0 secs.
Operations where demand exceeds capacity. Very long delays with queuing may be experienced on the minor street approach.

## RAMPS AND RAMP JUNCTIONS

## LEVEL OF SERVICE A

Maximum Density - $10 \mathrm{pc} / \mathrm{mi} / \mathrm{ln}$
Unrestricted operations with no noticeable turbulence in the ramp influence area.

## LEVEL OF SERVICE B

Maximum Density - $20 \mathrm{pc} / \mathrm{mi} / \mathrm{ln}$
Minimal levels of turbulence exist and speeds of vehicles in the influence area begin to decline.

## LEVEL OF SERVICE C

Maximum Density - $28 \mathrm{pc} / \mathrm{mi} / \mathrm{ln}$
Level of turbulence becomes noticeable as average speed within the influence area declines. Driving conditions are still relatively comfortable at this level.

## LEVEL OF SERVICE D

Maximum Density - $35 \mathrm{pc} / \mathrm{mi} / \mathrm{ln}$
Turbulence levels become intrusive. Queues may form on some high volume on-ramps but freeway operation remains stable.

## LEVEL OF SERVICE E

Maximum Density - >35 pc/mi/ln
Conditions approaching and reaching capacity. Speeds are reduced and turbulence of merging/diverging vehicles becomes intrusive to all vehicles in the influence area. Flow levels approach capacity limits and minor changes in demand can cause ramp and freeway queues to occur.

## LEVEL OF SERVICE F

Maximum Density - Demand flow exceeds limits

Unstable, or breakdown, operation. Approaching demand flows exceed the discharge capacity of the downstream freeway or ramp. Queues are visibly formed on the freeway and on-ramps and will continue to grow as long as the approaching demand exceeds the discharge capacity.

APPENDIX B
TRAFFIC FIGURES



DTS • Provident Intelligent Land Use

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White Plains, NY 10601
P: 914.428.0010
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LEGEND
00 - VPH-PEAK PM HOUR (5:00-6:00)

Existing Traffic Volumes
Pound Ridge Library
271 Westchester Ave
Pound Ridge, Westchester County, NY

Project No. 1043
Scale: N.T.S. July 2023

Figure No. 02


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Project No. 1043
Scale: N.T.S.

Pound Ridge Library
271 Westchester Ave
Pound Ridge, Westchester County, NY
No-Build Traffic Volumes


## APPENDIX C

## LOS SUMMARY TABLES

| TABLE C-1PEAK HOUR LEVEL OF SERVICE SUMMARY TABLEWestchester Avenue/Salem Drive \& Stone Hill road/Site Driveway |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: |
| APPROACH |  | PM Peak Hour |  |  |
|  |  | $\begin{gathered} 2023 \\ \text { EXISTING } \end{gathered}$ | $\begin{gathered} 2025 \\ \text { NO-BUILD } \end{gathered}$ | $\begin{gathered} 2025 \\ \text { BUILD } \end{gathered}$ |
|  |  | $\begin{gathered} \text { LOS } \\ \text { DELAY }(\mathrm{sec}) \end{gathered}$ | $\begin{gathered} \text { LOS } \\ \text { DELAY }(\mathrm{sec}) \end{gathered}$ | $\begin{gathered} \text { LOS } \\ \text { DELAY }(\mathrm{sec}) \end{gathered}$ |
| Westchester Avenue (RT 124) |  |  |  |  |
| NB | TOTAL | $\begin{gathered} \hline \mathrm{a} \\ 0.8 \end{gathered}$ | $\begin{gathered} \mathrm{a} \\ 0.7 \end{gathered}$ | $\begin{gathered} \hline \mathrm{a} \\ 0.8 \end{gathered}$ |
| Salem Road (RT 124) |  |  |  |  |
| SB | TOTAL | $\begin{gathered} \hline \mathrm{a} \\ 9.9 \end{gathered}$ | $\begin{gathered} \hline \mathrm{a} \\ 10.0 \\ \hline \end{gathered}$ | $\begin{gathered} \hline \mathrm{b} \\ 10.4 \\ \hline \end{gathered}$ |
| Stone Hill Road (RT 137) |  |  |  |  |
| EB | TOTAL | $\begin{gathered} \mathrm{a} \\ 4.2 \end{gathered}$ | $\begin{gathered} \mathrm{a} \\ 4.3 \end{gathered}$ | $\begin{gathered} \mathrm{a} \\ 4.2 \end{gathered}$ |
| Site Driveway |  |  |  |  |
| WB | TOTAL | $\begin{gathered} \mathrm{a} \\ 7.3 \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{a} \\ 7.9 \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{a} \\ 7.0 \\ \hline \end{gathered}$ |
| INTERSECTION |  | $\begin{gathered} \mathrm{a} \\ 3.4 \\ \hline \end{gathered}$ | $\begin{gathered} \mathrm{a} \\ 3.5 \end{gathered}$ | $\begin{gathered} \mathrm{a} \\ 3.6 \end{gathered}$ |

APPENDIX D
CAPACITY ANALYSIS

1: RT 124 \& RT 137/Driveway Performance by approach

| Approach | EB | WB | NB | SB | All |
| :--- | :---: | :---: | :---: | :---: | :---: |
| Denied Delay $(\mathrm{hr})$ | 0.0 | 0.0 | 0.1 | 0.0 | 0.1 |
| Denied Del/Veh (s) | 0.2 | 0.1 | 0.4 | 0.2 | 0.3 |
| Total Delay $(\mathrm{hr})$ | 0.2 | 0.0 | 0.1 | 0.4 | 0.8 |
| Total Del/Veh $(\mathrm{s})$ | 4.2 | 7.3 | 0.8 | 9.9 | 3.4 |

Total Network Performance

|  |  |
| :--- | :--- |
| Denied Delay (hr) | 0.1 |
| Denied Del/Veh (s) | 0.3 |
| Total Delay (hr) | 1.0 |
| Total Del/Veh (s) | 4.2 |

Intersection: 1: RT 124 \& RT 137/Driveway

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR |
| Maximum Queue (ft) | 68 | 31 | 93 |
| Average Queue (ft) | 18 | 13 | 42 |
| 95th Queue (ft) | 48 | 37 | 74 |
| Link Distance (ft) | 232 | 228 | 332 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Baa Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Network Summary |  |  |  |
| Network wide Queuing Penalty: 0 |  |  |  |

1: RT 124 \& RT 137/Driveway Performance by approach

| Approach | EB | WB | NB | SB | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Delay (hr) | 0.0 | 0.0 | 0.1 | 0.0 | 0.1 |
| Denied Del/Veh (s) | 0.2 | 0.1 | 0.4 | 0.3 | 0.3 |
| Total Delay (hr) | 0.2 | 0.0 | 0.1 | 0.4 | 0.8 |
| Total Del/Veh (s) | 4.3 | 7.9 | 0.7 | 10.0 | 3.5 |

Total Network Performance

|  |  |
| :--- | :--- |
| Denied Delay (hr) | 0.1 |
| Denied Del/Veh (s) | 0.3 |
| Total Delay (hr) | 0.9 |
| Total Del/Veh (s) | 4.3 |

Intersection: 1: RT 124 \& RT 137/Driveway

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR |
| Maximum Queue (ft) | 68 | 31 | 84 |
| Average Queue (ft) | 20 | 13 | 41 |
| 95th Queue (ft) | 49 | 37 | 69 |
| Link Distance (ft) | 232 | 228 | 332 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Baa Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Network Summary |  |  |  |
| Network wide Queuing Penalty: 0 |  |  |  |

1: RT 124 \& RT 137/Driveway Performance by approach

| Approach | EB | WB | NB | SB | All |
| :--- | ---: | ---: | ---: | ---: | ---: |
| Denied Delay (hr) | 0.0 | 0.0 | 0.1 | 0.0 | 0.1 |
| Denied Del/Veh (s) | 0.2 | 0.1 | 0.4 | 0.3 | 0.3 |
| Total Delay (hr) | 0.2 | 0.0 | 0.1 | 0.5 | 0.8 |
| Total Del/Veh (s) | 4.2 | 7.0 | 0.8 | 10.4 | 3.6 |

Total Network Performance

|  |  |
| :--- | :--- |
| Denied Delay (hr) | 0.1 |
| Denied Del/Veh (s) | 0.3 |
| Total Delay (hr) | 1.0 |
| Total Del/Veh (s) | 4.4 |

Intersection: 1: RT 124 \& RT 137/Driveway

| Movement | EB | WB | SB |
| :--- | ---: | ---: | ---: |
| Directions Served | LTR | LTR | LTR |
| Maximum Queue (ft) | 68 | 31 | 84 |
| Average Queue (ft) | 18 | 17 | 41 |
| 95th Queue (ft) | 47 | 42 | 73 |
| Link Distance (ft) | 232 | 228 | 332 |
| Upstream Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Storage Bay Dist (ft) |  |  |  |
| Storage Blk Time (\%) |  |  |  |
| Queuing Penalty (veh) |  |  |  |
| Network Summary |  |  |  |
| Network wide Queuing Penalty: 0 |  |  |  |

## APPENDIX E SPEED AND VOLUME DATA

## SPEED

Westchester Ave/Salem Ave S/O Pound Ridge Library Dwy
Day: Tuesday
Date: 6/20/2023

| Time | $<15$ | 15-19 | 20-24 | 25-29 | 30-34 | 35-39 | 40-44 | 45-49 | 50-54 | 55-59 | 60-64 | 65-69 | $70+$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 00:00 AM | 0 | 0 | 0 | 3 | 5 | 7 | 1 | 1 | 1 | 0 | 0 | 0 | 0 | 18 |
| 01:00 | 0 | 0 | 0 | 2 | 2 | 3 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 8 |
| 02:00 | 0 | 0 | 0 | 1 | 4 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 |
| 03:00 | 0 | 0 | 0 | 0 | 0 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 04:00 | 0 | 0 | 1 | 1 | 2 | 3 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 7 |
| 05:00 | 1 | 0 | 1 | 2 | 7 | 7 | 4 | 4 | 0 | 0 | 0 | 0 | 0 | 26 |
| 06:00 | 0 | 0 | 1 | 3 | 17 | 26 | 8 | 1 | 0 | 0 | 0 | 0 | 0 | 56 |
| 07:00 | 0 | 1 | 1 | 26 | 90 | 61 | 14 | 1 | 0 | 0 | 0 | 0 | 0 | 194 |
| 08:00 | 0 | 0 | 1 | 22 | 103 | 82 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 216 |
| 09:00 | 0 | 2 | 6 | 23 | 102 | 66 | 13 | 1 | 0 | 0 | 0 | 0 | 0 | 213 |
| 10:00 | 0 | 1 | 4 | 36 | 93 | 54 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 193 |
| 11:00 | 0 | 3 | 3 | 14 | 103 | 70 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 205 |
| 12:00 PM | 1 | 2 | 1 | 36 | 89 | 66 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 199 |
| 13:00 | 3 | 1 | 4 | 30 | 97 | 63 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 208 |
| 14:00 | 1 | 1 | 7 | 45 | 107 | 81 | 9 | 1 | 0 | 0 | 0 | 0 | 0 | 252 |
| 15:00 | 0 | 0 | 6 | 67 | 166 | 107 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 359 |
| 16:00 | 0 | 1 | 2 | 38 | 209 | 170 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 435 |
| 17:00 | 0 | 0 | 0 | 20 | 222 | 173 | 37 | 2 | 0 | 0 | 0 | 0 | 0 | 454 |
| 18:00 | 0 | 0 | 1 | 21 | 135 | 137 | 14 | 1 | 1 | 0 | 0 | 0 | 0 | 310 |
| 19:00 | 0 | 0 | 4 | 21 | 60 | 57 | 9 | 0 | 0 | 0 | 0 | 0 | 0 | 151 |
| 20:00 | 0 | 0 | 3 | 24 | 56 | 44 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 131 |
| 21:00 | 0 | 0 | 1 | 9 | 38 | 35 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 87 |
| 22:00 | 0 | 0 | 0 | 7 | 18 | 14 | 3 | 2 | 0 | 0 | 0 | 0 | 0 | 44 |
| 23:00 | 0 | 0 | 0 | 2 | 10 | 11 | 7 | 0 | 0 | 1 | 0 | 0 | 0 | 31 |
| Totals | 6 | 12 | 47 | 453 | 1735 | 1339 | 193 | 16 | 2 | 2 |  |  |  | 3805 |
| \% of Totals | 0\% | 0\% | 1\% | 12\% | 46\% | 35\% | 5\% | 0\% | 0\% | 0\% |  |  |  | 100\% |



| Street Name | Direction | Percentiles |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 15th | 50th | Average | 85th | 95th | ADT |
| Westchester Ave/Salem Ave | North Bound | 30 | 34 | 34 | 39 | 41 | 3805 |
| Westchester Ave/Salem Ave | South Bound | 26 | 30 | 30 | 34 | 37 | 3817 |

## SPEED

Westchester Ave/Salem Ave S/O Pound Ridge Library Dwy
Day: Tuesday
Date: 6/20/2023

City: Pound Ridge
Project \#: NY23_380018_001s

| Time | < 15 | 15-19 | 20-24 | 25-29 | 30-34 | 35-39 | 40-44 | 45-49 | 50-54 | 55-59 | 60-64 | 65-69 | $70+$ | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 00:00 AM | 0 | 1 | 0 | 2 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 9 |
| 01:00 | 0 | 0 | 0 | 0 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 2 |
| 02:00 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 0 |
| 03:00 | 0 | 0 | 0 | 2 | 0 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 3 |
| 04:00 | 0 | 0 | 2 | 3 | 7 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 14 |
| 05:00 | 0 | 0 | 1 | 17 | 26 | 3 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 49 |
| 06:00 | 0 | 0 | 11 | 59 | 91 | 12 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 174 |
| 07:00 | 0 | 2 | 14 | 179 | 221 | 41 | 1 | 1 | 0 | 0 | 0 | 0 | 0 | 459 |
| 08:00 | 0 | 12 | 46 | 220 | 249 | 31 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 558 |
| 09:00 | 4 | 8 | 24 | 127 | 142 | 25 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 331 |
| 10:00 | 0 | 1 | 9 | 82 | 94 | 12 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 199 |
| 11:00 | 2 | 6 | 18 | 57 | 81 | 18 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 184 |
| 12:00 PM | 0 | 0 | 25 | 89 | 85 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 211 |
| 13:00 | 3 | 0 | 22 | 70 | 80 | 10 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 186 |
| 14:00 | 0 | 2 | 20 | 77 | 89 | 18 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 210 |
| 15:00 | 1 | 6 | 22 | 85 | 99 | 12 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 225 |
| 16:00 | 0 | 2 | 13 | 121 | 105 | 18 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 259 |
| 17:00 | 2 | 0 | 10 | 122 | 162 | 22 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 322 |
| 18:00 | 0 | 4 | 10 | 55 | 88 | 18 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 177 |
| 19:00 | 0 | 0 | 2 | 35 | 44 | 21 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 103 |
| 20:00 | 0 | 0 | 5 | 20 | 25 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 55 |
| 21:00 | 0 | 0 | 0 | 16 | 17 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 41 |
| 22:00 | 0 | 0 | 1 | 13 | 9 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 25 |
| 23:00 | 0 | 0 | 1 | 4 | 9 | 5 | 2 | 0 | 0 | 0 | 0 | 0 | 0 | 21 |
| Totals | 12 | 44 | 256 | 1455 | 1731 | 296 | 22 | 1 |  |  |  |  |  | 3817 |
| \% of Totals | 0\% | 1\% | 7\% | 38\% | 45\% | 8\% | 1\% | 0\% |  |  |  |  |  | 100\% |
|  |  |  |  |  |  |  |  |  |  |  |  |  |  |  |
| AM Volumes | 6 | 30 | 125 | 748 | 919 | 145 | 8 | 1 | 0 | 0 | 0 | 0 | 0 | 1982 |
| \% AM | 0\% | 1\% | 3\% | 20\% | 24\% | 4\% | 0\% | 0\% |  |  |  |  |  | 52\% |
| AM Peak Hour | 09:00 | 08:00 | 08:00 | 08:00 | 08:00 | 07:00 | 05:00 | 07:00 |  |  |  |  |  | 08:00 |
| Volume | 4 | 12 | 46 | 220 | 249 | 41 | 2 | 1 |  |  |  |  |  | 558 |
| PM Volumes | 6 | 14 | 131 | 707 | 812 | 151 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 1835 |
| \% PM | 0\% | 0\% | 3\% | 19\% | 21\% | 4\% | 0\% |  |  |  |  |  |  | 48\% |
| PM Peak Hour | 13:00 | 15:00 | 12:00 | 17:00 | 17:00 | 17:00 | 14:00 |  |  |  |  |  |  | 17:00 |
| Volume | 3 | 6 | 25 | 122 | 162 | 22 | 4 |  |  |  |  |  |  | 322 |
| Directional Peak Periods All Speeds |  |  | AM 7-9 |  |  | NOON 12-2 |  |  | PM 4-6 |  |  | Off Peak Volumes |  |  |
|  |  |  | Volume 1017 | $\longleftrightarrow$ | $\begin{gathered} \% \\ 27 \% \end{gathered}$ | Volume 397 | $\longleftrightarrow$ | $\begin{gathered} \% \\ 10 \% \end{gathered}$ | Volume 581 | $\longleftrightarrow$ | $\begin{gathered} \% \\ 15 \% \end{gathered}$ | Volume 1822 |  | $\begin{gathered} \% \\ 48 \% \end{gathered}$ |


| Street Name | Direction | Percentiles |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 15th | 50th | Average | 85th | 95th | ADT |
| Westchester Ave/Salem Ave | North Bound | 30 | 34 | 34 | 39 | 41 | 3805 |
| Westchester Ave/Salem Ave | South Bound | 26 | 30 | 30 | 34 | 37 | 3817 |

## SPEED

Westchester Ave/Salem Ave S/O Pound Ridge Library Dwy
Day: Tuesday
Date: 6/20/2023

City: Pound Ridge
Project \#: NY23_380018_001

| Time | $<15$ | 15-19 | 20-24 | 25-29 | 30-34 | 35-39 | 40-44 | 45-49 | 50-54 | 55-59 | 60-64 | 65-69 | 70 + | Total |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| 00:00 AM | 0 | 1 | 0 | 5 | 11 | 7 | 1 | 1 | 1 | 0 | 0 | 0 | 0 | 27 |
| 01:00 | 0 | 0 | 0 | 2 | 4 | 3 | 0 | 0 | 0 | 1 | 0 | 0 | 0 | 10 |
| 02:00 | 0 | 0 | 0 | 1 | 4 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 6 |
| 03:00 | 0 | 0 | 0 | 2 | 0 | 2 | 1 | 0 | 0 | 0 | 0 | 0 | 0 | 5 |
| 04:00 | 0 | 0 | 3 | 4 | 9 | 5 | 0 | 0 | 0 | 0 | 0 | 0 | 0 | 21 |
| 05:00 | 1 | 0 | 2 | 19 | 33 | 10 | 6 | 4 | 0 | 0 | 0 | 0 | 0 | 75 |
| 06:00 | 0 | 0 | 12 | 62 | 108 | 38 | 9 | 1 | 0 | 0 | 0 | 0 | 0 | 230 |
| 07:00 | 0 | 3 | 15 | 205 | 311 | 102 | 15 | 2 | 0 | 0 | 0 | 0 | 0 | 653 |
| 08:00 | 0 | 12 | 47 | 242 | 352 | 113 | 8 | 0 | 0 | 0 | 0 | 0 | 0 | 774 |
| 09:00 | 4 | 10 | 30 | 150 | 244 | 91 | 14 | 1 | 0 | 0 | 0 | 0 | 0 | 544 |
| 10:00 | 0 | 2 | 13 | 118 | 187 | 66 | 6 | 0 | 0 | 0 | 0 | 0 | 0 | 392 |
| 11:00 | 2 | 9 | 21 | 71 | 184 | 88 | 14 | 0 | 0 | 0 | 0 | 0 | 0 | 389 |
| 12:00 PM | 1 | 2 | 26 | 125 | 174 | 78 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 410 |
| 13:00 | 6 | 1 | 26 | 100 | 177 | 73 | 11 | 0 | 0 | 0 | 0 | 0 | 0 | 394 |
| 14:00 | 1 | 3 | 27 | 122 | 196 | 99 | 13 | 1 | 0 | 0 | 0 | 0 | 0 | 462 |
| 15:00 | 1 | 6 | 28 | 152 | 265 | 119 | 13 | 0 | 0 | 0 | 0 | 0 | 0 | 584 |
| 16:00 | 0 | 3 | 15 | 159 | 314 | 188 | 15 | 0 | 0 | 0 | 0 | 0 | 0 | 694 |
| 17:00 | 2 | 0 | 10 | 142 | 384 | 195 | 41 | 2 | 0 | 0 | 0 | 0 | 0 | 776 |
| 18:00 | 0 | 4 | 11 | 76 | 223 | 155 | 16 | 1 | 1 | 0 | 0 | 0 | 0 | 487 |
| 19:00 | 0 | 0 | 6 | 56 | 104 | 78 | 10 | 0 | 0 | 0 | 0 | 0 | 0 | 254 |
| 20:00 | 0 | 0 | 8 | 44 | 81 | 49 | 4 | 0 | 0 | 0 | 0 | 0 | 0 | 186 |
| 21:00 | 0 | 0 | 1 | 25 | 55 | 43 | 2 | 2 | 0 | 0 | 0 | 0 | 0 | 128 |
| 22:00 | 0 | 0 | 1 | 20 | 27 | 16 | 3 | 2 | 0 | 0 | 0 | 0 | 0 | 69 |
| 23:00 | 0 | 0 | 1 | 6 | 19 | 16 | 9 | 0 | 0 | 1 | 0 | 0 | 0 | 52 |
| Totals | 18 | 56 | 303 | 1908 | 3466 | 1635 | 215 | 17 | 2 | 2 |  |  |  | 7622 |
| \% of Totals | 0\% | 1\% | 4\% | 25\% | 45\% | 21\% | 3\% | 0\% | 0\% | 0\% |  |  |  | 100\% |


| AM Volumes | 7 | 37 | 143 | 881 | 1447 | 526 | 74 | 9 | 1 | 1 | 0 | 0 | 0 | 3126 |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
| \% AM | 0\% | 0\% | 2\% | 12\% | 19\% | 7\% | 1\% | 0\% | 0\% | 0\% |  |  |  | 41\% |
| AM Peak Hour | 09:00 | 08:00 | 08:00 | 08:00 | 08:00 | 08:00 | 07:00 | 05:00 |  | 01:00 |  |  |  | 08:00 |
| Volume | 4 | 12 | 47 | 242 | 352 | 113 | 15 | 4 | 1 | 1 |  |  |  | 774 |
| PM Volumes | 11 | 19 | 160 | 1027 | 2019 | 1109 | 141 | 8 | 1 | 1 | 0 | 0 | 0 | 4496 |
| \% PM | 0\% | 0\% | 2\% | 13\% | 26\% | 15\% | 2\% | 0\% | 0\% | 0\% |  |  |  | 59\% |
| PM Peak Hour | 13:00 | 15:00 | 15:00 | 16:00 | 17:00 | 17:00 | 17:00 | 17:00 | 18:00 | 23:00 |  |  |  | 17:00 |
| Volume | 6 | 6 | 28 | 159 | 384 | 195 | 41 | 2 | 1 | 1 |  |  |  | 776 |
| Directional Peak Periods All Speeds |  |  | AM 7-9 |  |  | NOON 12-2 |  |  | PM 4-6 |  |  | Off Peak Volumes |  |  |
|  |  |  | Volume | $\rightarrow$ | $\begin{gathered} \% \\ 19 \% \end{gathered}$ | Volume |  | $\begin{gathered} \% \\ 11 \% \end{gathered}$ | Volume $1470$ | $\longrightarrow$ | $\begin{gathered} \% \\ 19 \% \end{gathered}$ | Volume $3921$ | $\longleftrightarrow$ | $\begin{gathered} \% \\ 51 \% \end{gathered}$ |


| Street Name | Direction | Percentiles |  |  |  |  |  |
| :---: | :---: | :---: | :---: | :---: | :---: | :---: | :---: |
|  |  | 15th | 50th | Average | 85th | 95th | ADT |
| Westchester Ave/Salem Ave | Summary | 27 | 32 | 32 | 37 | 40 | 7622 |

## VOLUME

Westchester Ave/Salem Ave S/O Pound Ridge Library Dwy

Day: Tuesday
Date: 6/20/2023

City: Pound Ridge
Project \#: NY23_380018_001


